

## Prestress Losses

References PCI Design Handbook, 8th Edition

Design File Heavy IT Beam

Description Calculation of prestress losses using the lump sum method present in the PCI Design Handbook.

### Geometry

Height of precast section	$h_{pre} := 32 \text{ in}$
Width of stem	$w_{stem} := 24 \text{ in}$
Thickness of ledge	$t_{ledge} := 12 \text{ in}$
Width of ledge	$w_{ledge} := 8 \text{ in}$
Gross Cross Section Area	$A_g := 960 \text{ in}^2$
Composite Cross Section Area	$A_c := 1124 \text{ in}^2$
Area of Topping	$A_t := 2.75 \text{ in} \cdot 68 \text{ in} = 187 \text{ in}^2$
Centroid Location of the Gross Cross Section	$cg_y := 14 \text{ in}$
Centroid Location of the Composition Section	$cg_{yc} := 16.659 \text{ in}$
Height of the member	$h := 32 \text{ in}$
Gross Section Moment of Inertia	$I_{zz.g} := 83200 \text{ in}^4$
Composite Section Moment of Inertia	$I_{zz.c} := 132753.7 \text{ in}^4$

### Material Properties

Initial Elast Modulus	$E_{ci} := 3586 \text{ ksi}$
30-Day Elastic Modulus	$E_{cf} := 4695 \text{ ksi}$
Relative Humidity	$R_h := 70$

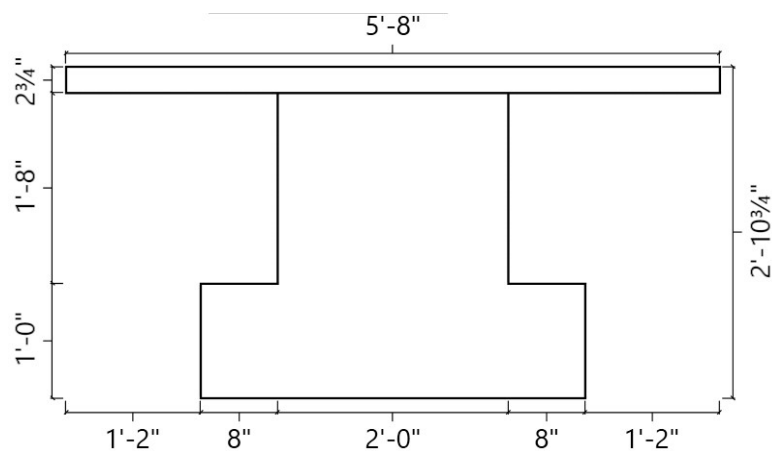


Figure 1: Concrete Geometry of Composite Precast IT Beam

### Prestress Properties

Number of strand in rows 1-4	$num_{row1} := 16$
	$num_{row2} := 14$
	$num_{row3} := 2$

Centroid of strand in rows 1-4 measured from the bottom

$$num_{row4} := 2$$

$$cg_{row1} := 3 \text{ in}$$

$$cg_{row2} := 5 \text{ in}$$

$$cg_{row3} := 10 \text{ in}$$

$$cg_{row4} := 30 \text{ in}$$

Area of individual strand

$$A_{strand} := 0.167 \text{ in}^2$$

Strand elastic modulus

$$E_{ps} := 29000 \text{ ksi}$$

Strand ultimate Stress

$$f_{pu} := 270 \text{ ksi}$$

Initial strand stress

$$f_{pi} := 0.75 \cdot f_{pu} = 202.5 \text{ ksi}$$

Total area of strand

$$A_{ps} := A_{strand} \cdot (num_{row1} + num_{row2} + num_{row3} + num_{row4}) = 5.678 \text{ in}^2$$

Centroid of strand

$$cg_{st} := \frac{num_{row1} \cdot cg_{row1} + num_{row2} \cdot cg_{row2} + num_{row3} \cdot cg_{row3} + num_{row4} \cdot cg_{row4}}{num_{row1} + num_{row2} + num_{row3} + num_{row4}} = 5.824 \text{ in}$$

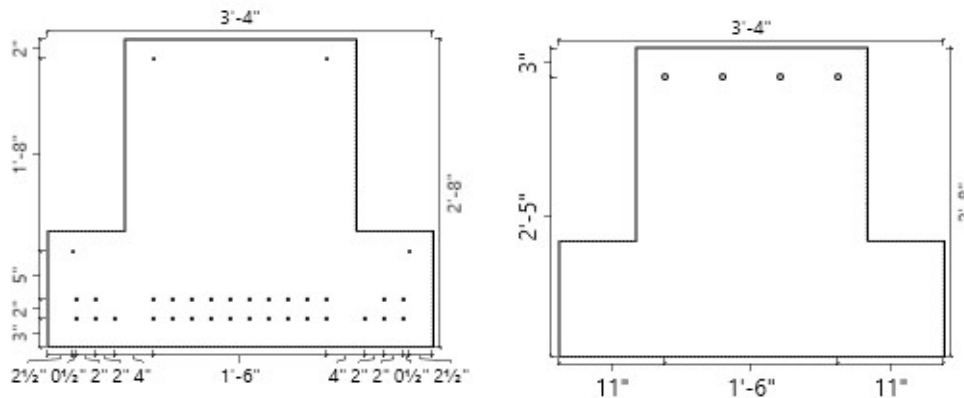


Figure 2: Reinforcement Locations of Strand (Left) and Rebar (Right)

## Loading

Moment due to self weight

$$M_{SW} := 249.389 \text{ kip ft}$$

Moment due to non-composite dead load

$$M_{Dnc} := 498.778 \text{ kip ft}$$

Moment due to topping weight

$$M_{Dt} := 48.5789 \text{ kip ft}$$

Moment due to composite dead load

$$M_{Dc} := 997.556 \text{ kip ft}$$

Moment due to live load

$$M_{LL} := 748.167 \text{ kip ft}$$

## Elastic Shortening

Always 1.0 for prestressed components

$$K_{es} := 1.0$$

Strand eccentricity

$$e := cg_y - cg_{st} = 8.1765 \text{ in}$$

Always 0.9 for prestressed components

$$K_{cir} := 0.9$$

New compressive stress at centroid of strand (Equation 5-100)

$$f_{cir} := K_{cir} \cdot \left( \frac{A_{ps} \cdot f_{pi}}{A_g} + \frac{A_{ps} \cdot f_{pi} \cdot e^2}{I_{zz.g}} \right) - \frac{M_{SW} \cdot e}{I_{zz.g}} = 1615.3461 \text{ psi}$$

Elastic shortening (Equation 5-99)

$$ES := K_{es} \cdot \frac{E_{ps}}{E_{ci}} \cdot f_{cir} = 13063.3123 \text{ psi}$$

**Creep**

2.0 for normalweight concrete

$$K_{cr} := 2.0$$

Stress in concrete at centroid of strand due to super imposed permanent loads

Non composite loads

$$f_{c ds . nc} := \frac{(M_{Dnc} + M_{Dt}) \cdot e}{I_{zz.g}} = 645.4972 \text{ psi}$$

Composite loads

$$f_{c ds . c} := \frac{M_{Dc} \cdot (cg_{yc} - cg_{st})}{I_{zz.c}} = 977.0565 \text{ psi}$$

Total Stress

$$f_{c ds} := f_{c ds . nc} + f_{c ds . c} = 1622.5537 \text{ psi}$$

Creep (Equation 5-101)

$$CR := K_{cr} \cdot \frac{E_{ps}}{E_{cf}} \cdot (f_{cir} - f_{c ds}) = -89.0397 \text{ psi}$$

Creep is strictly positive and will use 0 a lower bound.

$$CR := 0 \text{ psi}$$

**Shrinkage**

Perimeter of the precast section

$$Perimeter := 2 \cdot w_{stem} + 2 \cdot h + 4 \cdot w_{ledge} = 144 \text{ in}$$

Volume to surface ratio

$$VS_{ratio} := \frac{A_g}{Perimeter} = 6.6667 \text{ in}$$

1.0 for prestressed components

$$K_{sh} := 1.0$$

Shrinkage (Equation 5-103)

$$SH := \left(8.2 \cdot 10^{-6}\right) \cdot K_{sh} \cdot E_{ps} \cdot \left(1 - \frac{0.06 \cdot VS_{ratio}}{\text{in}}\right) \cdot (100 - R_h) = 4280.4 \text{ psi}$$

**Relaxation**

From table 5.8.1

$$K_{re} := 5000 \text{ psi}$$

From table 5.8.1

$$J := 0.040$$

Coefficient (Equation 5-105)

$$C := \frac{\frac{f_{pi}}{f_{pu}}}{0.21} \cdot \left( \frac{\frac{f_{pi}}{f_{pu}}}{0.9} - 0.55 \right) = 1.0119$$

Relaxation (Equation 5-104)

$$RE := C \cdot \left( K_{re} - J \cdot (SH + CR + ES) \right) = 4357.5164 \text{ psi}$$

**Live Load Regain**

Stress at the centroid of strand due to live load

$$f_{c ll} := - \frac{M_{LL} \cdot (cg_{yc} - cg_{st})}{I_{zz.c}} = -732.7924 \text{ psi}$$

Total live load regain

$$LR := \frac{E_{ps}}{E_{cf}} \cdot f_{c ll} = -4526.3 \text{ psi}$$

**Total Losses**

Total losses

$$TL := ES + CR + SH + RE + LR = 17174.9286 \text{ psi}$$

Percent prestress loss

$$Loss := \frac{TL}{f_{pi}} = 8.4814 \%$$